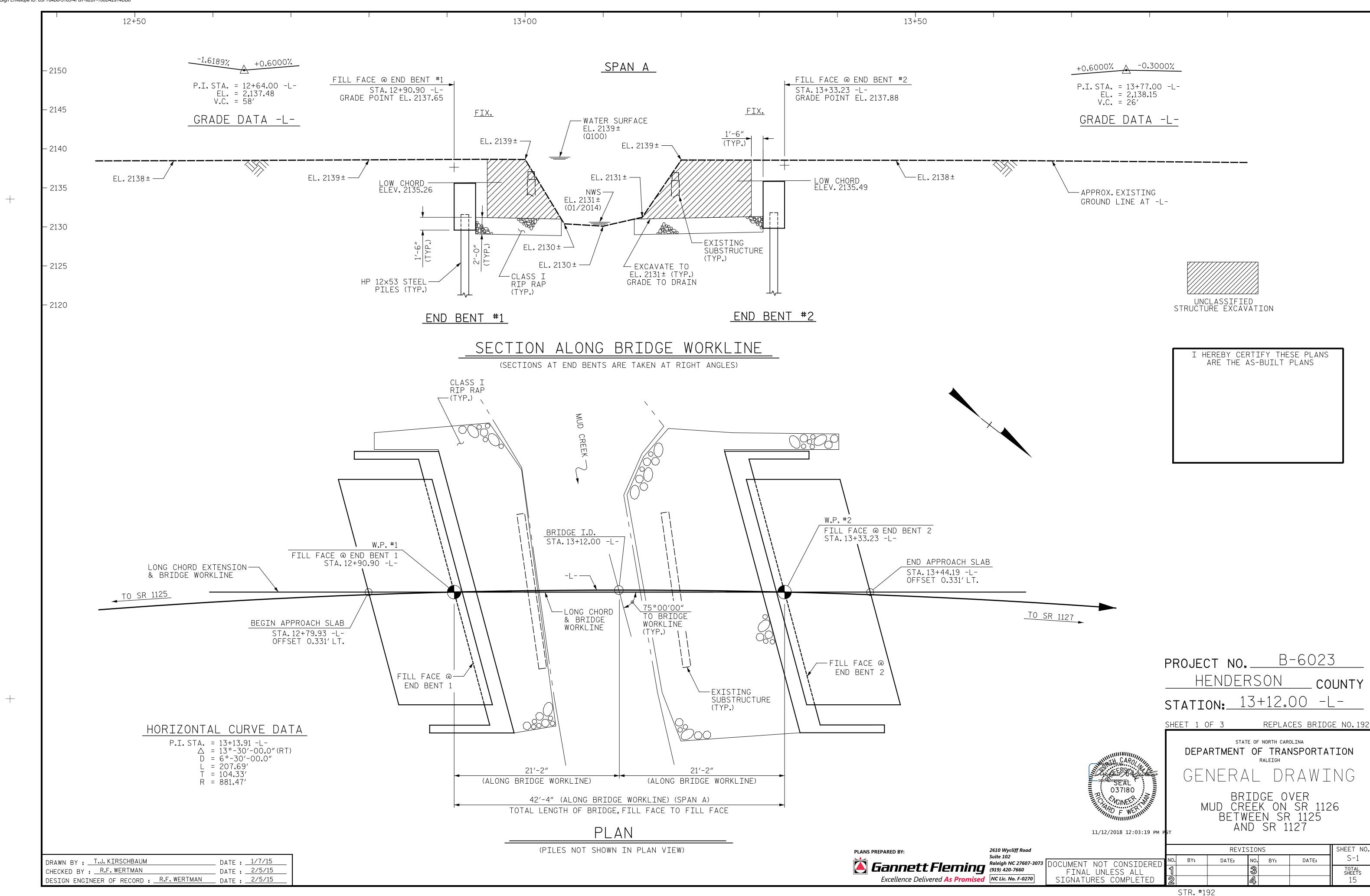
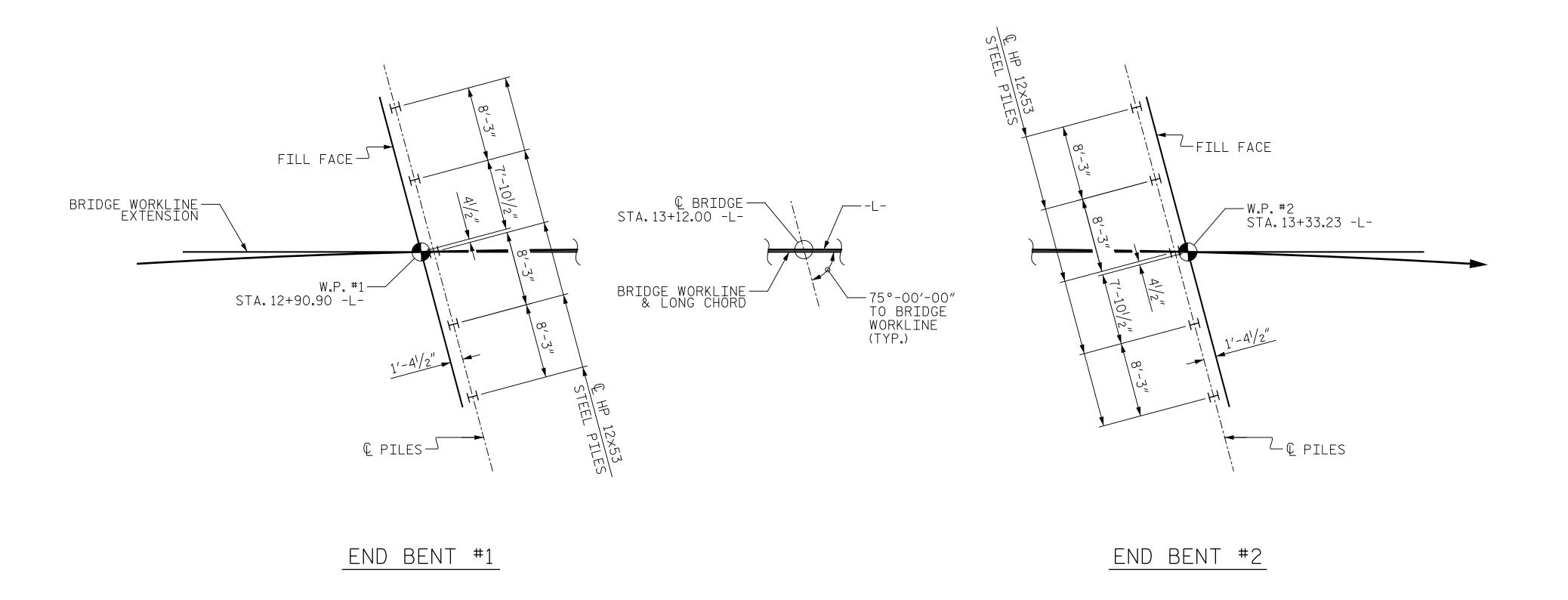
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+



FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE.

# NOTES:

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 78 TONS PER PILE.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 78 TONS PER PILE.

DRIVE LEFT PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 130 TONS PER PILE.

PILE EXCAVATION IS REQUIRED TO INSTALL CENTER AND RIGHT SIDE PILES AT END BENT NO.1. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 2119.0 FT.FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATION.

PILE EXCAVATION IS REQUIRED TO INSTALL ALL PILES AT END BENT NO.2.EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 2119.0 FT. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATION.

CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT END BENT NO.1 AND END BENT NO.2. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. B-6023

HENDERSON COUNTY

STATION: 13+12.00 -L-

STATE OF NORTH CAROLINA

SHEET 2 OF 3



DEPARTMENT OF TRANSPORTATION
RALEIGH

GEBF6470.
SEAL
37180

DEPARTMENT OF TRANSPORTATION
RALEIGH

CONTRACTOR OVER

BRIDGE OVER MUD CREEK ON SR 1126 BETWEEN SR 1125 AND SR 1127

PLANS PREPARED BY:

Cannett Fleming

Excellence Delivered As Promised

2610 Wycliff Road
Suite 102
Raleigh NC 27607-3073
(919) 420-7660

NC Lic. No. F-0270

2610 Wycliff Road
Suite 102
Raleigh NC 27607-3073
(919) 420-7660

NC Lic. No. F-0270

DOCUMENT NOT CONSIDER
FINAL UNLESS ALL
SIGNATURES COMPLETE

			REV]	SION	S		SHEET NO
USTDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
ALL	1			3			TOTAL SHEETS
PLETED	2			4			15

DRAWN BY: T.J. KIRSCHBAUM

CHECKED BY: R.F. WERTMAN

DATE: 1/7/15

DESIGN ENGINEER OF RECORD: R.F. WERTMAN

DATE: 2/24/15

BENCH MARK #2: NAVD88 NAIL IN BASE OF 12"WALNUT, -L- STA.13+44.94, 35.77' RIGHT, EL. 2139.46 WOODS **PU**₱ROPOSED GUARDRAIL (TYP. AS SHOWN) PUE — -EXISTING & PAY ITEM) <del>(</del>3 STRUCTURE र्दुः (TYP.) TOE PROTECTION-TO SR 1125 TO SR 1127 -75°-00′-00″ (TO BRIDGE WORKLINE)  $\leftarrow$  (TYP.) BRIDGE I.D. - PDE - 13+12. PDE L-BM #2 WOODS FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS LOCATION SKETCH

# NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT. EACH SIDE OF BRIDGE WORK LINE AT END BENT 1 AND END BENT 2 AS DIRECTED BY THE ENGINEER, THIS WORK WILL BE PAID AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF ONE SPAN AT 18'-6", WITH A CLEAR ROADWAY OF 19'-2" ON TIMBER JOISTS ON END BENTS WITH TIMBER CAPS ON TIMBER PILES LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 EVALUATING SCOUR AT BRIDGES".

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

# HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION

DRAINAGE AREA BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION

= 850 C.F.S. = 10 YRS. = 2137**.**5 = 4.4 SQ. MI. = 1,700 C.F.S.

= 2138.93

# OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEVATION

= 950 C.F.S. = 10+ YRS. = 2138.1

	TOTAL BILL OF MATERIAL															
	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP STEI	12 X 53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS I (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0"X 1'- PRESTRESS CONCRETI CORED SLA
	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EACH	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO. LIN.
SUPERSTRUCTURE							LUMP SUM					80,25			LUMP SUM	10 400
END BENT NO.1			21.0	9.0	LUMP SUM	31,5		3,125	5	5	75		35	40		
END BENT NO.2			37.0	13.0	LUMP SUM	31,5		3,125	5	5	75		65	70		
TOTAL	LUMP SUM	LUMP SUM	58.0	22.0	LUMP SUM	63.0	LUMP SUM	6,250	10	10	150	80.25	100	110	LUMP SUM	10 400

PROJECT NO. B-6023 HENDERSON COUNTY STATION: 13+12.00 -L-

SHEET 3 OF 3

037180

11/19/2018 4:01:28 PM PST

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING

BRIDGE OVER MUD CREEK ON SR 1126 BETWEEN SR 1125 AND SR 1127

DATE : 1/7/15 \_\_ DATE : <u>2/6/15</u> DESIGN ENGINEER OF RECORD : \_ R.F. WERTMAN \_\_ DATE : \_ 2/6/15

CHECKED BY : R.F. WERTMAN

Gannett Fleming (919) 420-7660 Excellence Delivered **As Promised** NC Lic. No. F-0270

OCUMENT NOT CONSIDERE FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS SHEET NO. NO. BY: S-3 BY: DATE: TOTAL SHEETS

### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT RATING DISTI FACT DIST FACT DIS. FAC $S \vdash A$ HL-93(Inv)N/A 1.573 1.75 0.274 1.78 40′ EL 19.482 0.62 1.6 40′ EL 1.948 0.80 0.274 1.57 40′ EL 19.482 HL-93(0pr) N/A 2.078 1.35 0.274 2.31 40′ EL 19.482 0.62 2.08 40′ 1.948 N/A EL DESIGN LOAD 36.000 1.873 67.422 1.75 0.274 2.23 19.482 0.62 1.87 40′ 1.948 0.80 0.274 1.96 HS-20(Inv) 40′ EL EL 40′ 19.482 EL RATING 36.000 2.428 0.274 EL 19.482 0.62 40′ HS-20(0pr) 87.4 2.9 40′ 2.43 EL 1.948 N/A \_\_\_ 0.62 0.80 0.274 13.500 3.642 49.163 40′ EL 19.482 4.8 40′ EL 7.793 3.64 40′ 19.482 SNSH 0.274 5.16 EL 15.586 SNGARBS2 20.000 2.995 59.897 0.274 4.24 40′ EL 0.62 3.65 40′ EL 7.793 0.80 0.274 2.99 40′ EL 19.482 15.586 0.80 SNAGRIS2 22.000 2.951 64.924 0.274 4.14 40′ EL 0.62 3.49 40′ EL 7.793 0.274 2.95 40′ 15.586 EL 27.250 49.605 2.58 40′ EL 19.482 0.62 40′ 7.793 0.80 0.274 1.82 40′ 19.482 SNCOTTS3 0.274 2.42 EL EL 57.231 2.32 0.62 2.18 0.80 0.274 SNAGGRS4 34.925 1.639 40′ EL 19.482 40′ EL 7.793 1.64 40′ 19.482 0.274 EL 35.550 SNS5A 1.594 56.664 0.274 2.26 40′ EL 19.482 0.62 2.3 40′ EL 7.793 0.80 0.274 1.59 40′ 19.482 EL 2.15 0.62 2.16 40′ 0.80 0.274 1.52 SNS6A 39.950 1.517 60.592 0.274 40′ EL 19.482 EL 1.948 40′ EL 19.482 1.948 0.80 SNS7B 42.000 1.446 60.751 0.274 2.05 40′ EL 19.482 0.62 2.2 40′ EL 0.274 1.45 40′ 19.482 EL LEGAL LOAD 0.274 2.65 0.62 2.52 1.948 0.80 0.274 1.87 TNAGRIT3 33.000 1.866 40′ EL 19.482 40′ EL 40′ 19.482 61.588 EL RATING 33.075 0.80 0.274 TNT4A 62.524 0.274 2.68 40′ EL 19.482 0.62 2.38 40′ EL 7.793 1.89 40′ 19.482 EL 2.27 40′ 0.80 0.274 41.600 1.603 66.702 0.274 40′ EL 19.482 0.62 2.34 EL 7.793 1.60 40′ TNT6A EL 19.482 1.948 42.000 69.051 40′ EL 19.482 0.62 2.17 40′ 0.80 0.274 TNT7A 1.644 0.274 2.33 EL 1.64 40′ EL 19.482 0.80 TNT7B 42.000 1.681 70.584 0.274 2.38 40′ EL 19.482 0.62 2.09 40′ 1.948 0.274 1.68 40′ 19.482 EL EL 15.586 0.274 TNAGRIT4 43.000 70.321 0.274 2.31 40′ EL 0.62 2 40′ EL 1.948 0.80 1.64 40' 19.482 EL 1.513 TNAGT5A 45.000 0.274 40′ 19.482 0.62 2.08 40′ 1.948 0.80 0.274 1.51 19.482 EL

19.482

DESIGN RATING FACTORS

LOAD FACTORS:

 $\gamma_{\mathsf{DC}}$ LIMIT STATE 1.25 STRENGTH I SERVICE III | 1.00 | 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

# COMMENTS:

19.482

EL

(#) CONTROLLING LOAD RATING

 $\langle 1 \rangle$  DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$  DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

# GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-6023 HENDERSON COUNTY

STATION: 13+12.00 -L-



OCUMENT NOT CONSIDERE

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

LRFR SUMMARY FOR 40' CORED SLAB UNIT 75° SKEW

(NON-INTERSTATE TRAFFIC)

SHEET NO. REVISIONS S-4 NO. BY: DATE: BY: DATE: TOTAL SHEETS

\_RFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: T.J.KIRSCHBAUM DATE: 12/1/14 CHECKED BY: R.F.WERTMAN DATE: -

TNAGT5B

45.000

66.159

DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

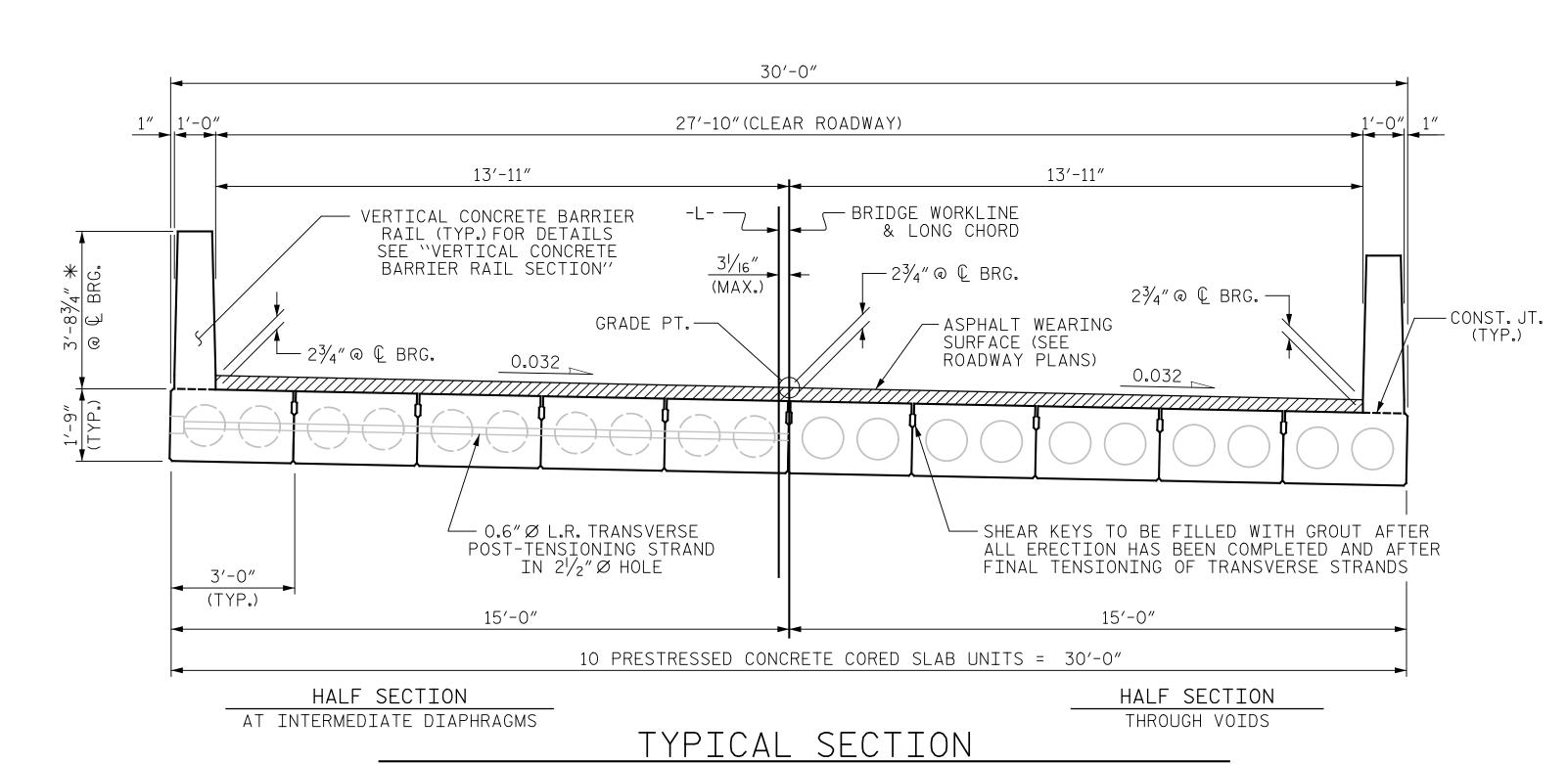


1.948

0.80

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED, I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

+



\*-THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

 $\sim 2^{1/2}$  Ø DOWEL HOLE

VOIDS

- ELASTOMERIC BEARING PAD

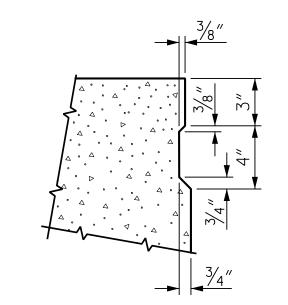
-SEE "END BENT" SHEETS FOR DETAILS

FIXED END

SECTION AT END BENT

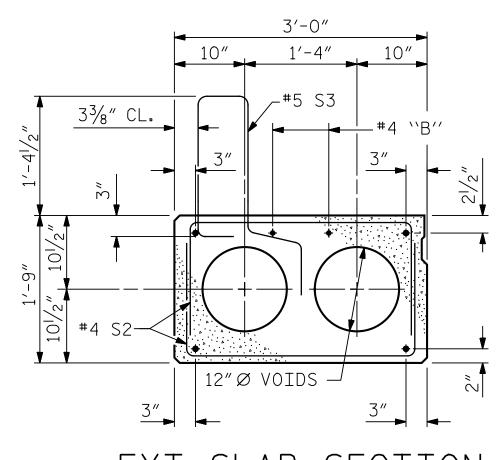
ASPHALT

WEARING SURFACE—



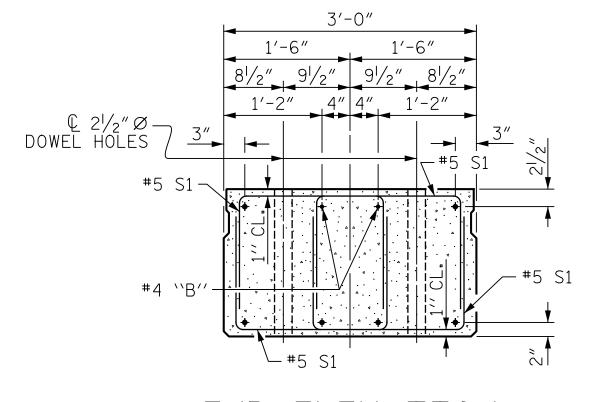
# SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



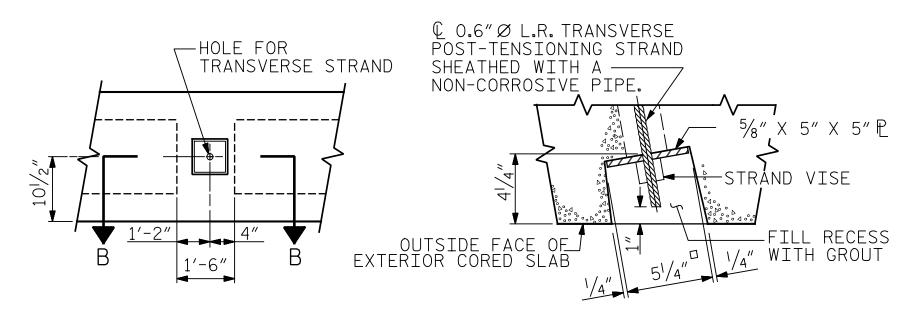
EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)



# ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



**ELEVATION VIEW** 

SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 12/1/14 CHECKED BY: R.F. WERTMAN DATE: 2/6/15 DRAWN BY: DGE 5/09
CHECKED BY: BCH 6/09
REV. 8/14 MAA/TMG

SEE "BRIDGE — APPROACH SLAB"

SHEET FOR DETAILS

2 LAYERS OF 30 LB.

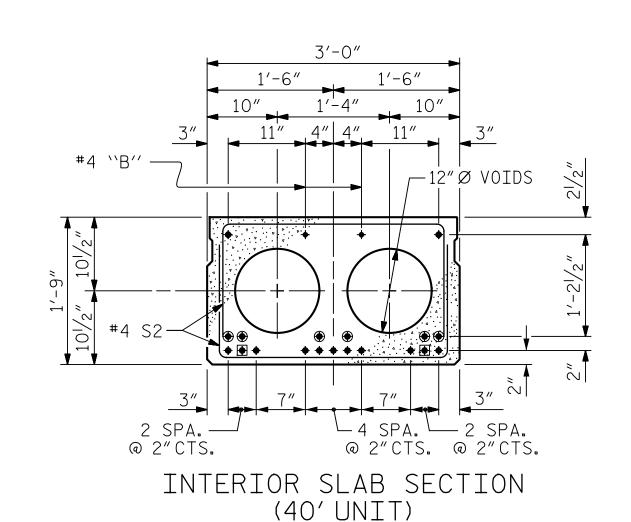
 $1\frac{1}{2}$ " Ø BACKER ROD-

ROOFING FELT TO PREVENT BOND.

© BEARING & #6 DOWELS



THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED, I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

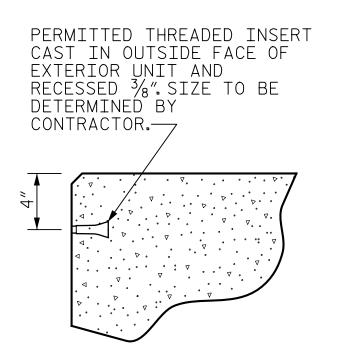


### 0.6"Ø LOW RELAXATION STRAND LAYOUT

(13 STRANDS REQUIRED)

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- ( OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

# DEBONDING LEGEND



# THREADED INSERT DETAIL

B-6023 PROJECT NO.

> HENDERSON COUNTY

13+12.00 -L-

SHEET 1 OF 3



DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 1'-9" PRESTRESSED CONCRETE

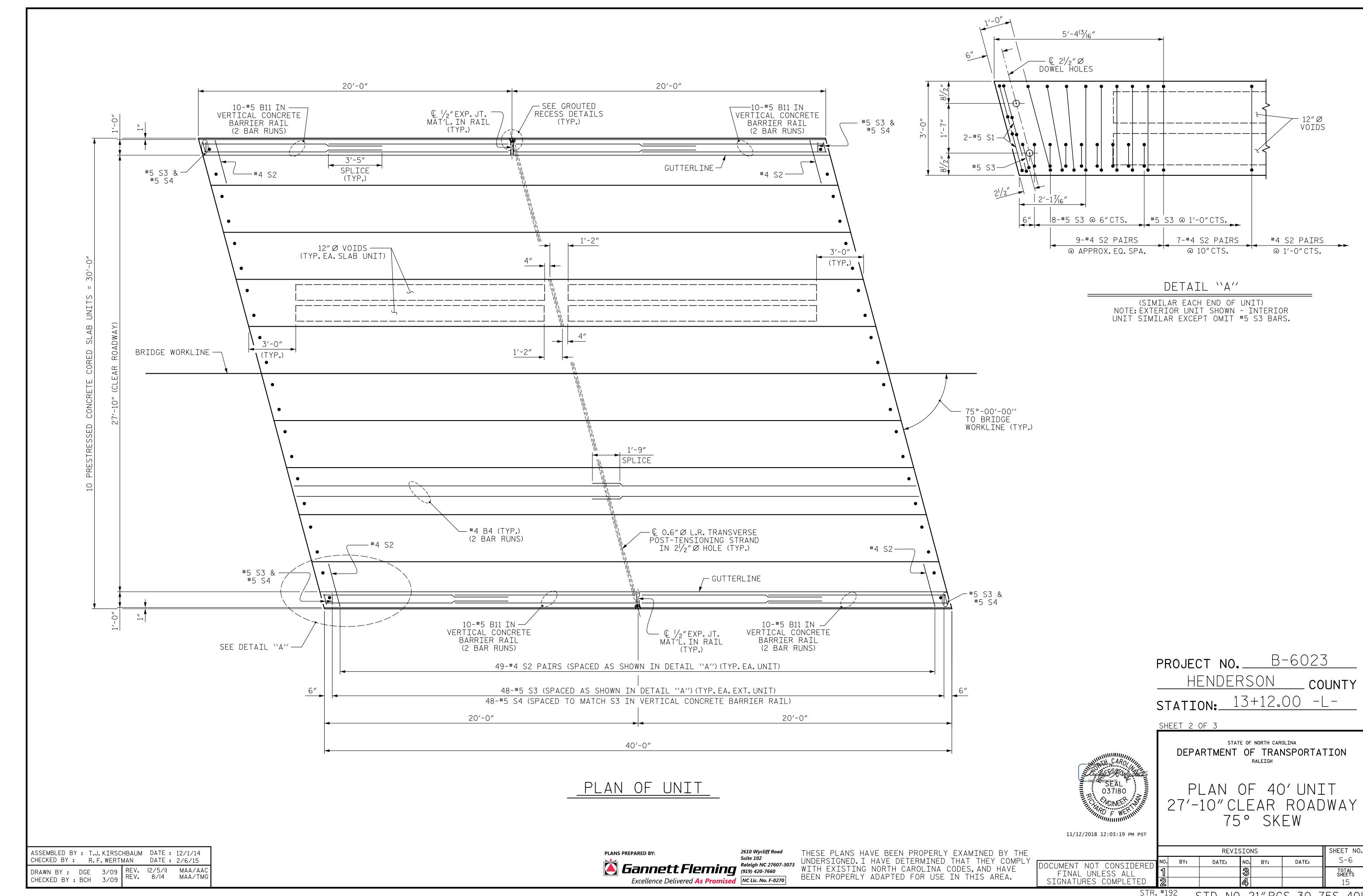
STATE OF NORTH CAROLINA

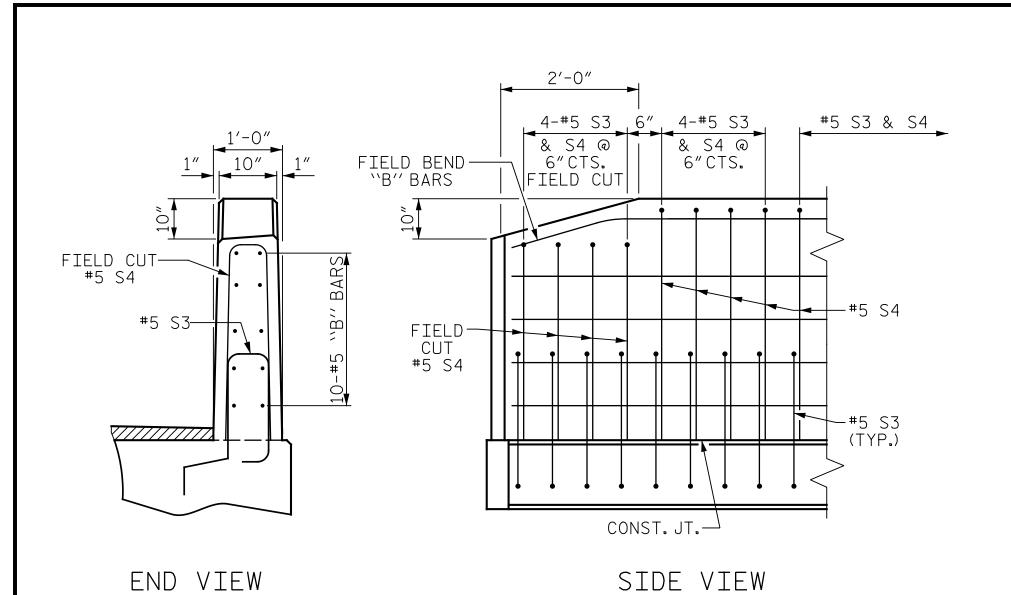
CORED SLAB UNIT 75° SKEW

SHEET NO REVISIONS S-5 NO. BY: DATE: BY: DATE: DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED TOTAL SHEETS

STD. NO. 21" PCS2\_30\_75S

STR. #192





END OF RAIL DETAILS

—#5 S4

#5 S3 (SEE

"PLAN OF UNIT"

VERTICAL CONCRETE BARRIER RAIL SECTION

FOR SPACING)

1'-0"

2"CL.MIN.

AS

3'-8¾" SEE ''GUTTERLINE THICKNESS & RAIL HE

# DEAD LOAD DEELECTION AND CAMBER

DEAD LOAD DEFLECTION AT	ND CAMPER
	3'-0" × 1'-9"
40'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	7⁄8″ ੈ
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	<sup>1</sup> /8″ <b>↓</b>
FINAL CAMBER	3/4″ ♠
★ INCLUDES FUTURE WEARING SURF	FACE

** INCLUDES FUTURE WEARING SURFACE								
CORED SLABS REQUIRED								
	NUMBER	LENGTH	TOTAL LENGTH					
40'UNIT								
EXTERIOR C.S.	2	40'-0"	80'-0"					
INTERIOR C.S.	8	40'-0"	320′-0″					
TOTAL	10		400'-0"					

	BILL OF MATERIAL FOR ONE 40'CORED SLAB UNIT									
				EXTERI	OR UNIT	INTERI	OR UNIT			
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT			
В4	4	#4	STR	20'-9"	55	20′-9″	55			
S1	8	#5	3	4'-3"	35	4'-3"	35			
S2	98	#4	3	5′-4″	349	5′-4″	349			
* S3	50	#5	1	5′-7″	291					
REINFO	ORCING :	STEEL	LE	BS.	439		439			
₩ EPOX	(Y COATE	ED								
REIN	REINFORCING STEEL			3S.	291					
5000	5000 P.S.I. CONCRETE C			DS.	5.8		5.8			
0.6"Ø	L.R. STR	ANDS		۷٥.	13		13			

ALL BAR DIMENSIONS ARE OUT TO OUT

BAR TYPES

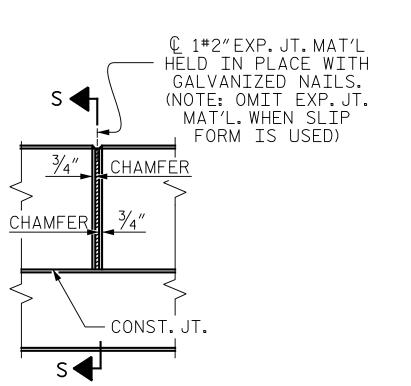
S2 2'-8"

© BEARING PAD —

6"

73/4"

CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
40' UNITS	4000



GRADE 270 S	TRANDS
	0.6″Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

# ELEVATION AT EXPANSION JOINTS

SECTION S-S

AT DAM IN OPEN JOINT

(THIS IS TO BE USED ONLY

WHEN SLIP FORM IS USED)

# ELASTOMERIC BEARING DETAILS

-BEARING PAD - TYPE I -

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

FIXED END

(TYPE I - 20 REQ'D)

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL								
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT		
	40' UNIT							
<b>★</b> B11	80	80	#5	STR	11'-8"	973		
<del>*</del> S4	100	100	#5	2	7'-2"	747		
₩ EPOX	(Y COATED REINFORCING STEEL			LBS.		1720		
CLASS	AA CONCRETE		CU.YDS.	1	10.2			
TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT. 80						80.25		

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2^{1/2}$   $^{\prime\prime}$   $\varnothing$  DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER, SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS, A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR. SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
27'-10"CLEAR ROADWAY	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
40'UNITS	2"	3′-8″

B-6023 PROJECT NO. \_\_\_ HENDERSON COUNTY 13+12.00 -L-

SHEET 3 OF 3



DEPARTMENT OF TRANSPORTATION 11/12/2018 12:03:19 PM PST

STANDARD 3'-0" X 1'-9" PRESTRESSÉD CONCRETE CORED SLAB UNIT 75° SKEW

STATE OF NORTH CAROLINA

SHEET NO REVISIONS S-7 NO. BY: DATE: BY: DATE: OCUMENT NOT CONSIDERE FINAL UNLESS ALL SIGNATURES COMPLETED

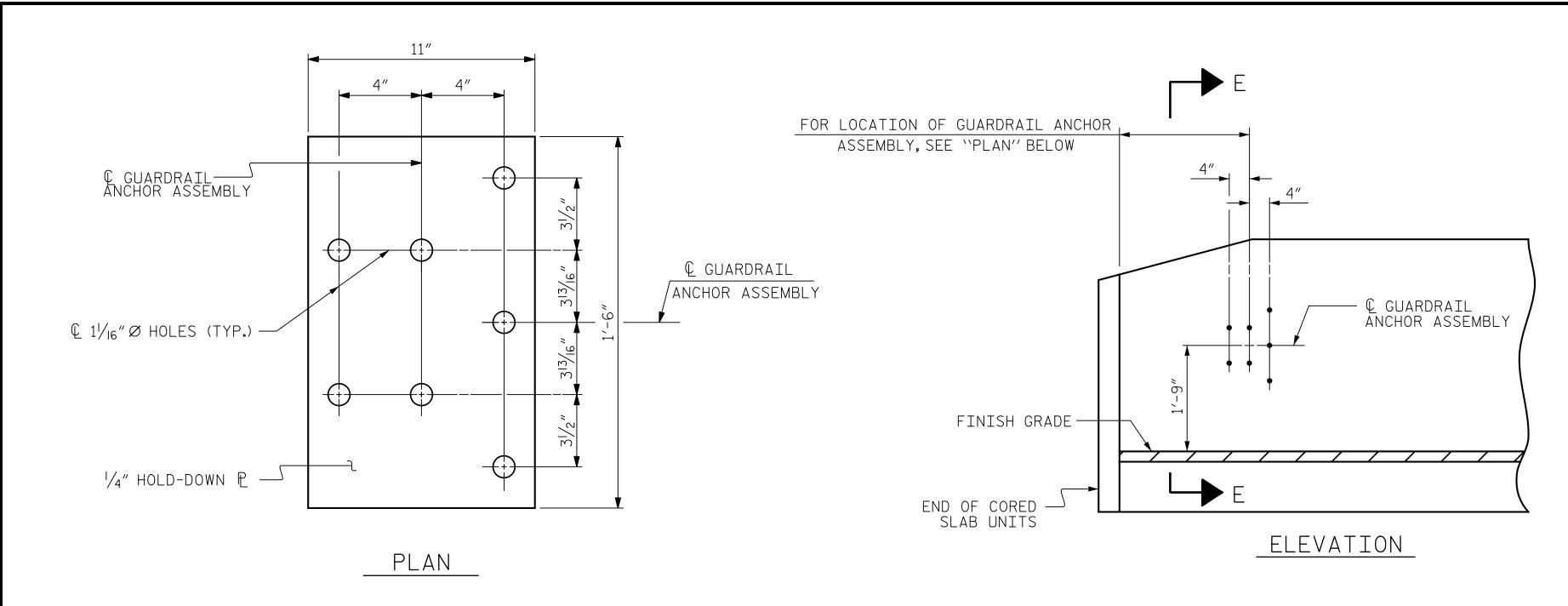
ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 12/1/14 CHECKED BY: R.F. WERTMAN DATE: 2/6/15 DRAWN BY: DGE 5/09 REV. 5/18 MAA/THC CHECKED BY : BCH 6/09

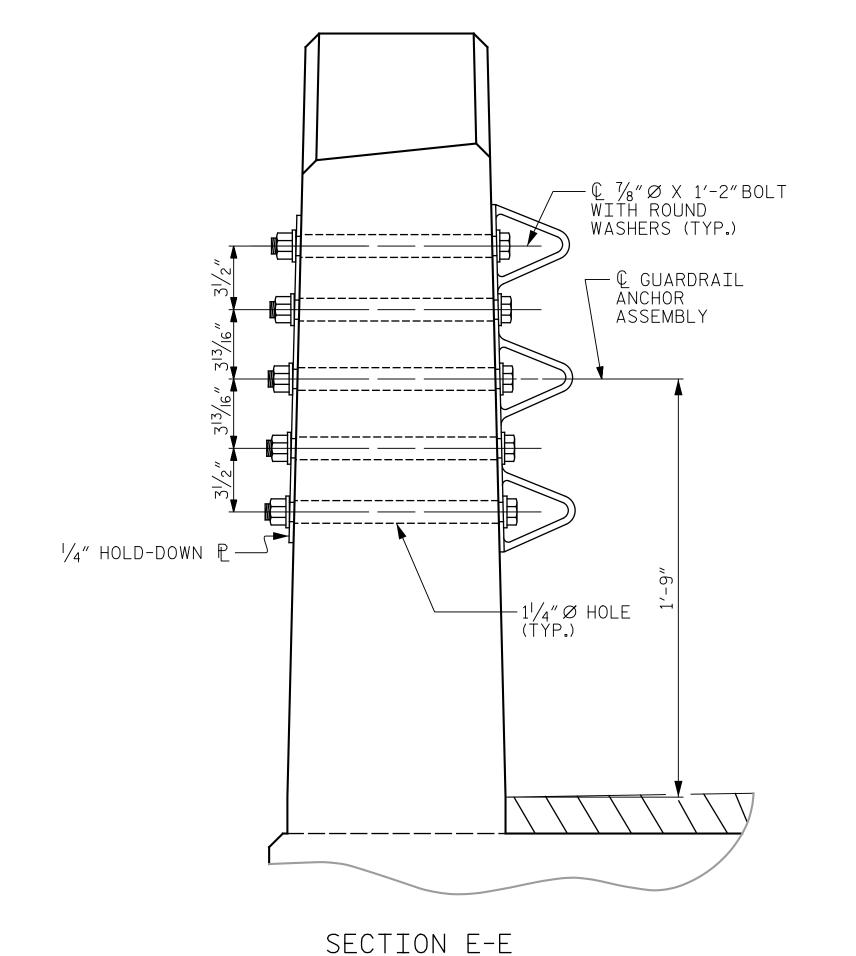
CONST. JT. —



Raleigh NC 27607-3073

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.





MAA/GM

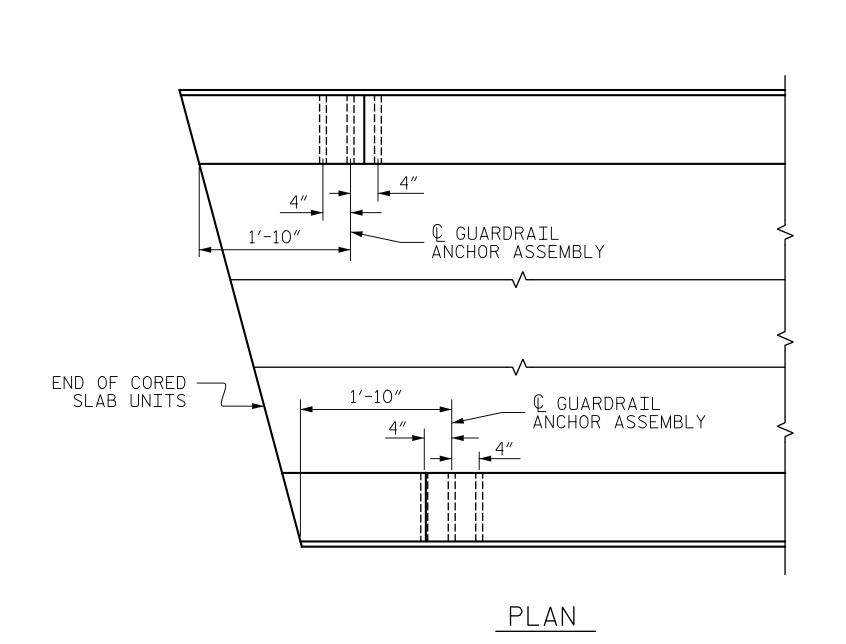
MAA/THC

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 12/1/14 CHECKED BY: R.F. WERTMAN DATE: 2/6/15

DRAWN BY: MAA 5/10

CHECKED BY : GM 5/10

GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR

# NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4"HOLD DOWN PLATE AND 7 -  $\frac{7}{8}$ " \alpha BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

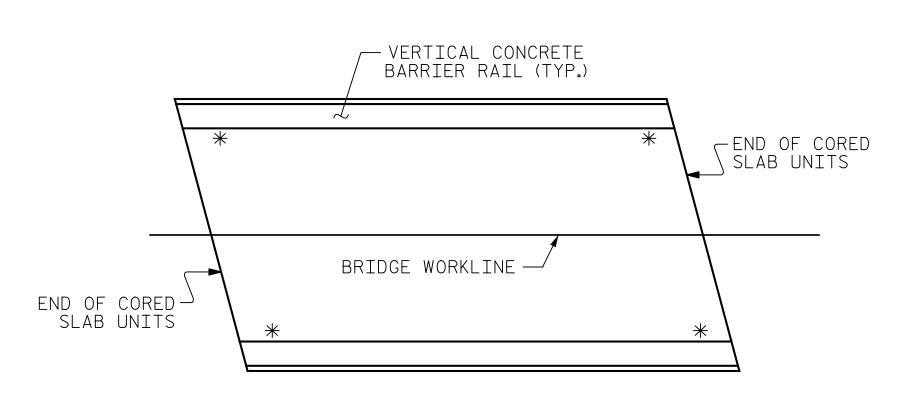
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ "  $\varnothing$  HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED, ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



# SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-6023 HENDERSON COUNTY STATION: 13+12.00 -L-



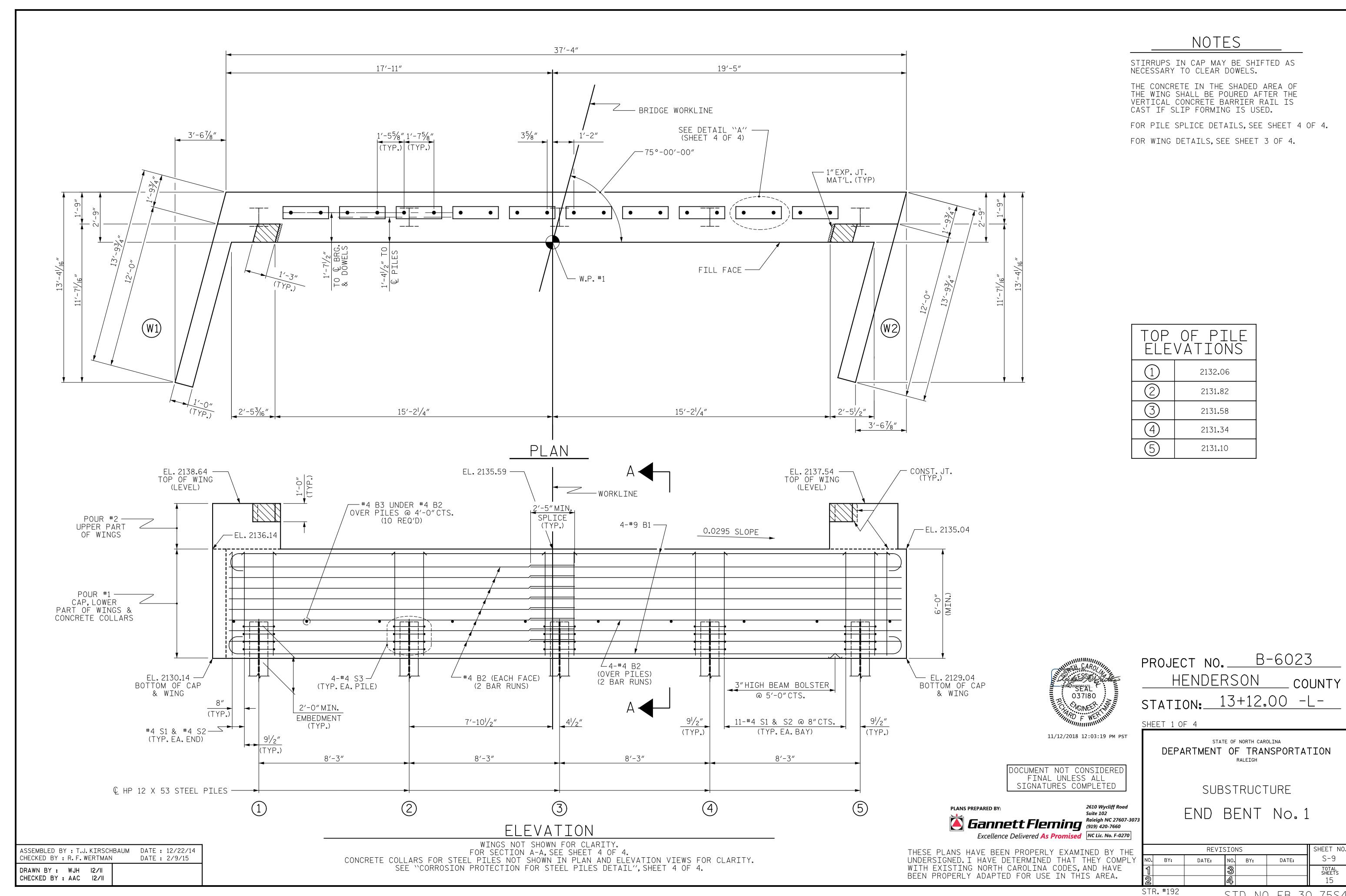
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

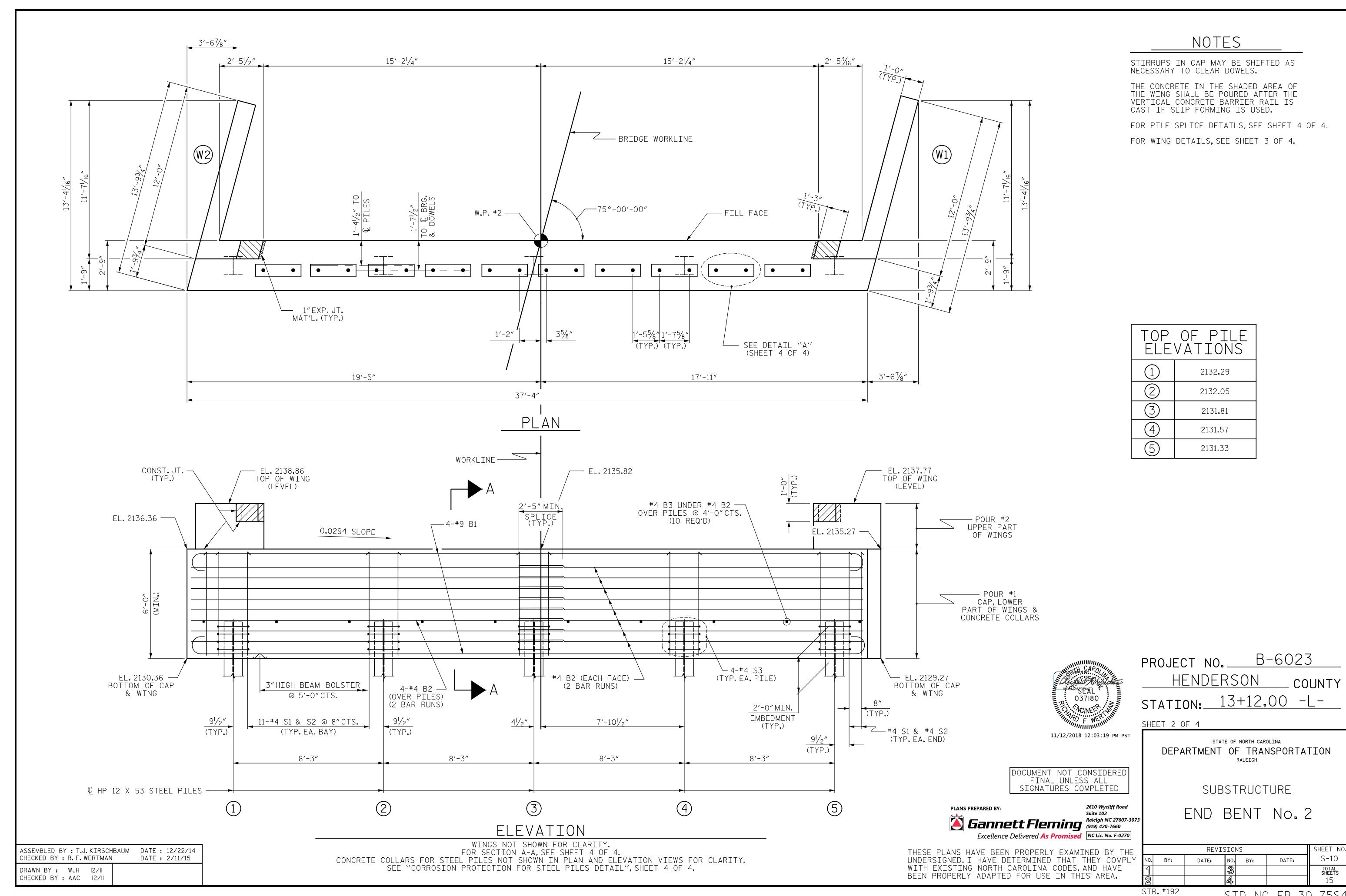
GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL

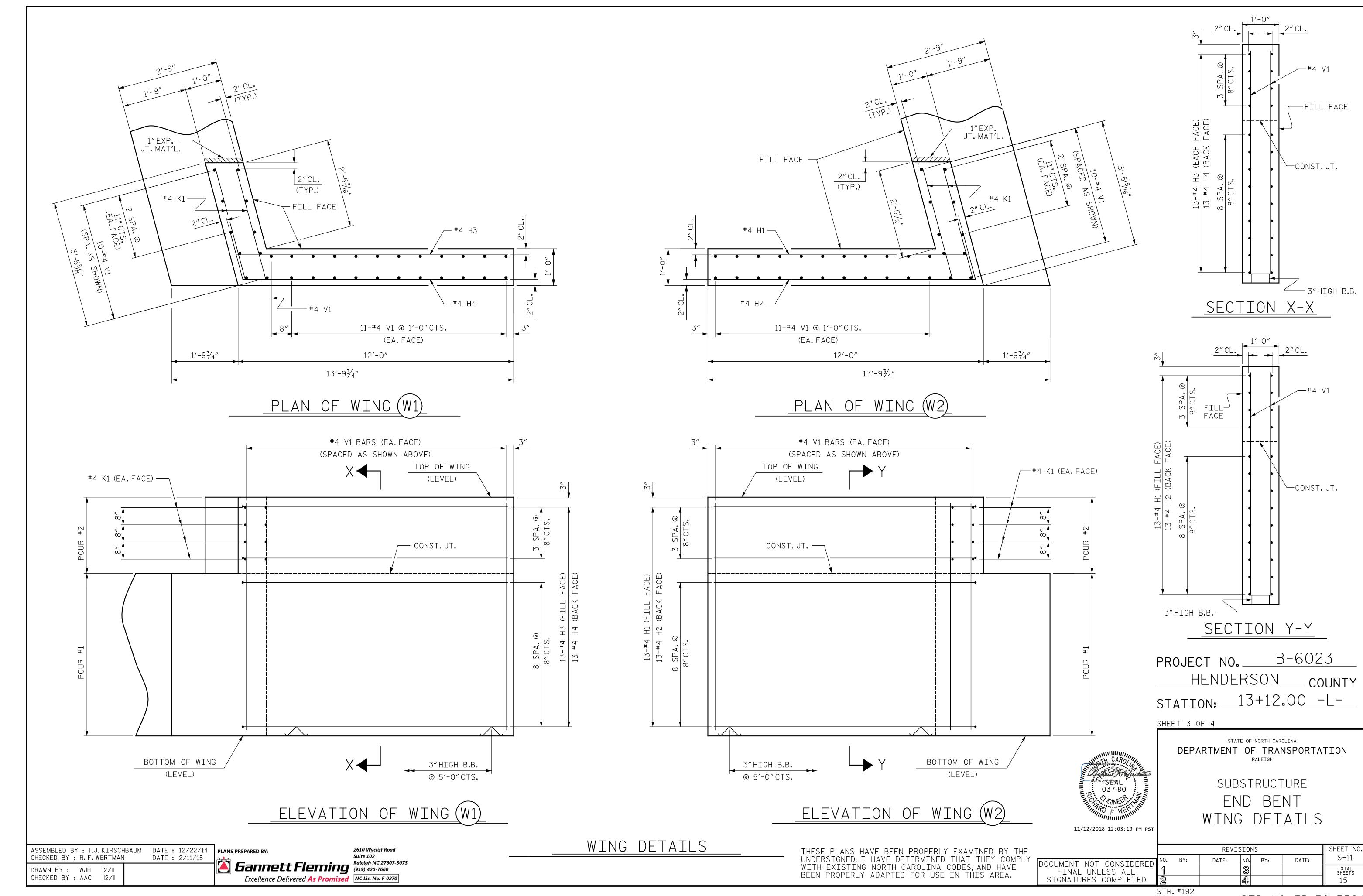
11/12/2018 12:03:19 PM P

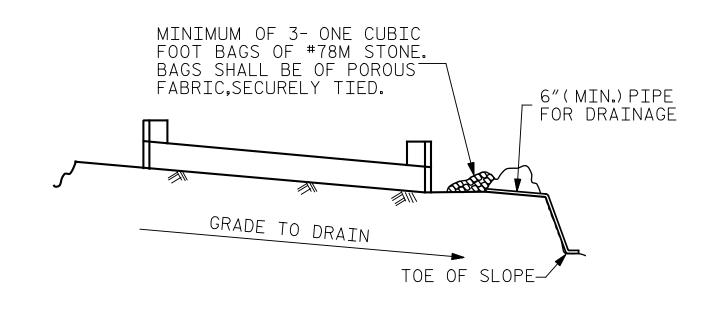
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STR. #192







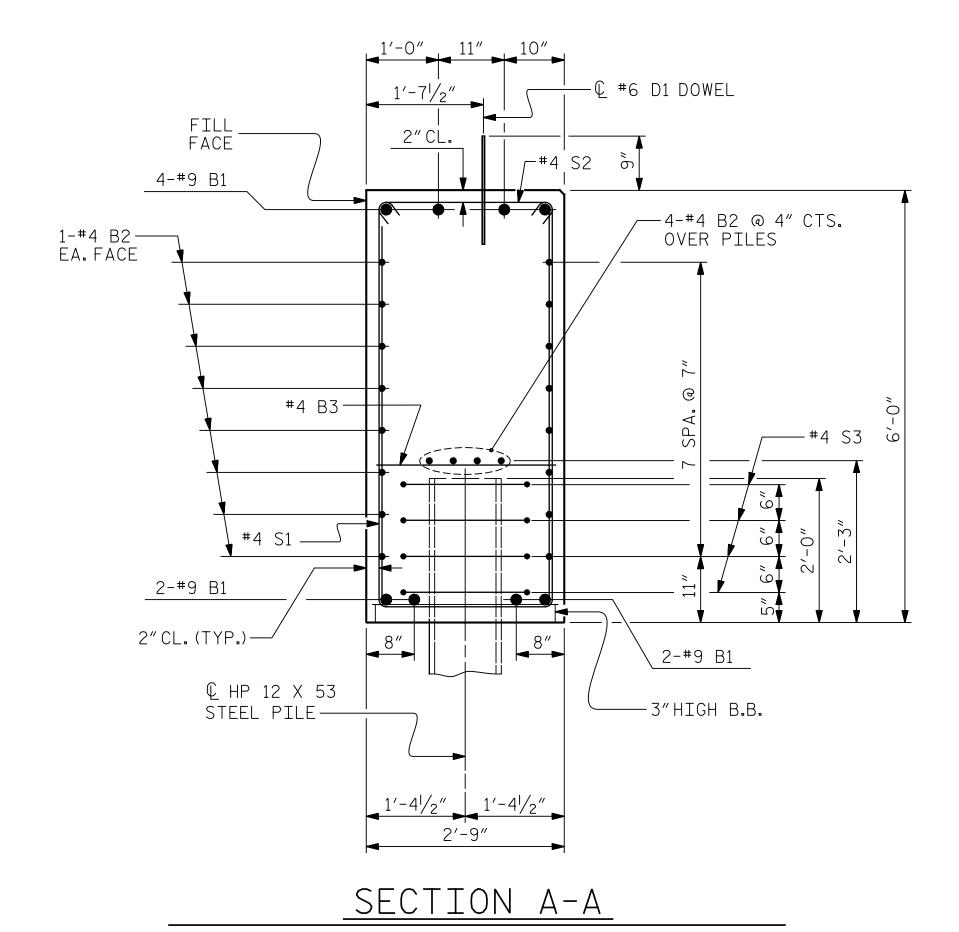


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

# TEMPORARY DRAINAGE AT END BENT



(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL."

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 12/22/14

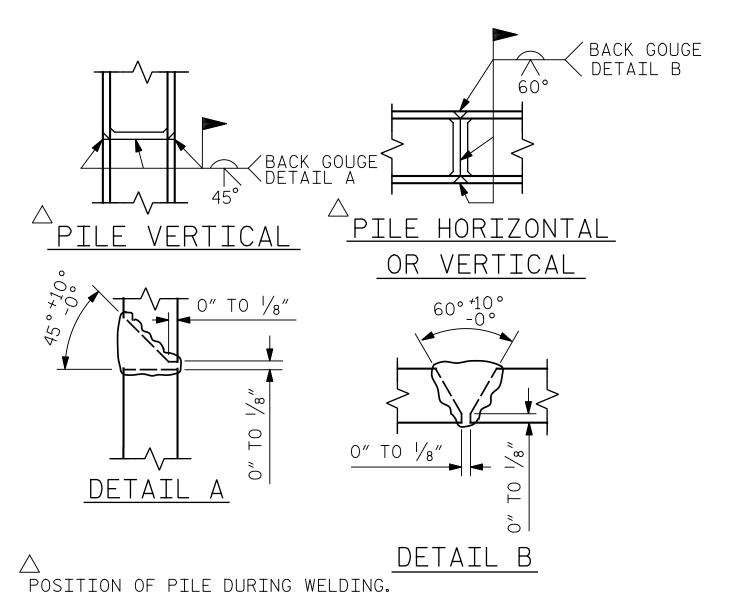
DATE: 2/11/15

MAA/THC

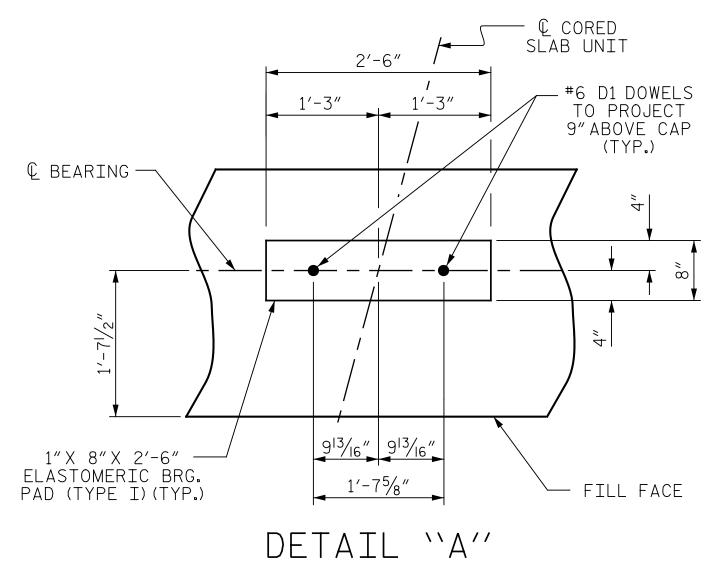
CHECKED BY : R.F. WERTMAN

DRAWN BY: WJH 12/II

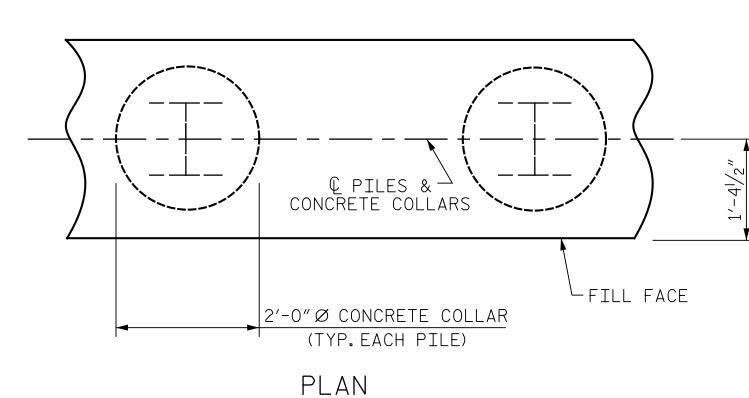
CHECKED BY : AAC | 12/11



# PILE SPLICE DETAILS



(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



CONCRETE — COLLAR -BOTTOM OF CAP © HP 12 X 53 STEEL PILE 2'-0" ELEVATION

CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

FOR ONE END BENT BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 39′-4″ #9 | 1 | #4 | STR | В2 19′-9″ 528 36′-10″ 40 В3 #4 | STR | 10 2′-5″ 16 11'-5" D1 20 #6 | STR | 1′-6″ 45 11'-7" H1 #4 12'-1" 105 Н2 #4 12′-3″ 106 Н3 #4 12′-6″ 109 Н4 #4 107 3 12′-4″ 13 #4 | STR | 3'-1" 33 16 14′-5″ 462 S1 48 #4 S2 #4 102 48 3′-2″ -1'-3" LAP S3 | 20 #4 6′-6″ 87 6 V1 | 65 | #4 | STR | 8′-2″ 355 2'-5" REINFORCING STEEL 3,125 LBS (FOR ONE END BENT) 2′-5″ CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT) POUR #1 CAP, LOWER PART 28.8 C.Y OF WINGS & COLLARS

BILL OF MATERIAL

BAR TYPES

9.0 LIN.FT. NOT IN SOIL

NO: 5

11'-10"

11'-8"

1'-8"Ø

IN SOIL

NOT IN SOIL

PILE DRIVING EQUIPMENT

SETUP FOR

HP 12 X 53 STEEL PILES

ALL BAR DIMENSIONS ARE OUT TO OUT. POUR #2 UPPER PART OF 2.7 C.Y WINGS END BENT No.1 END BENT No. 2 HP 12 X 53 STEEL PILES HP 12 X 53 STEEL PILES LIN.FT. = 75 LIN. FT. = 75 | TOTAL CLASS A CONCRETE 31.5 C.Y No. = 5 END BENT No.1 END BENT No. 2 PILE EXCAVATION PILE EXCAVATION 21.0 LIN. FT. IN SOIL 37.0 LIN. FT. PILE EXCAVATION PILE EXCAVATION

13.0 LIN.FT

NO: 5

PILE DRIVING EQUIPMENT

SETUP FOR

037180

11/12/2018 12:03:19 PM PST

HP 12 X 53 STEEL PILES

B-6023 PROJECT NO. \_\_\_ HENDERSON COUNTY 13+12.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

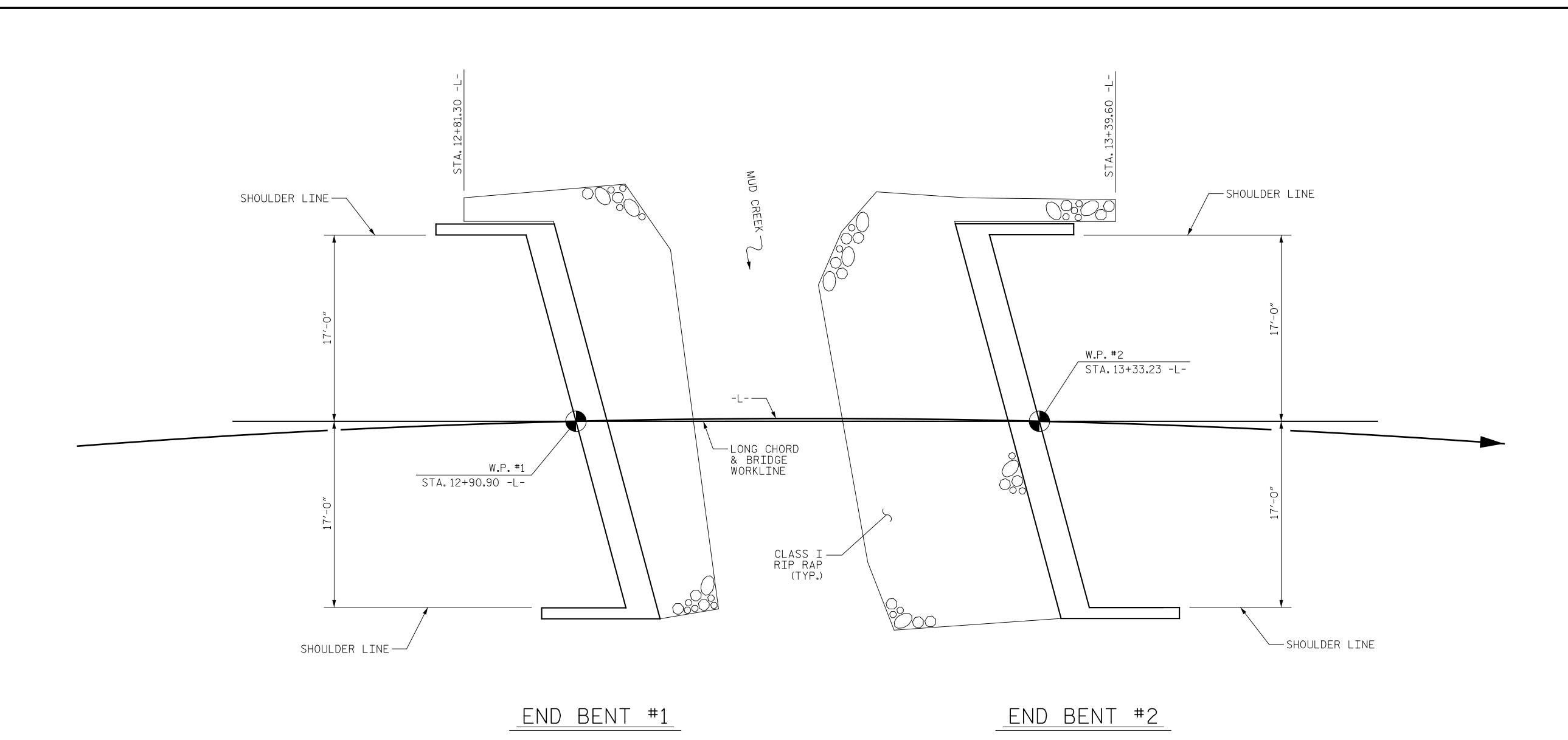
SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

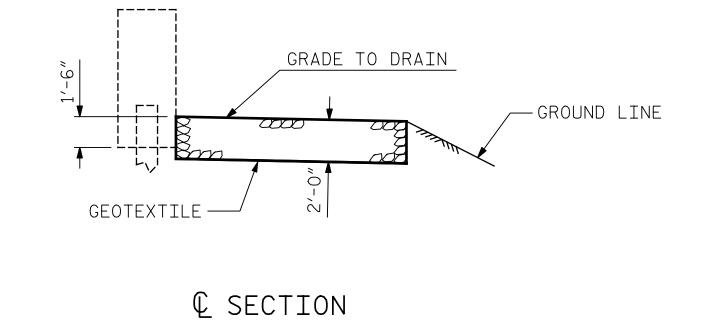
REVISIONS SHEET NO S-12 NO. BY: DATE: BY: DATE: OOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED SHEETS

STD. NO. EB\_30\_75S4

**Gannett Fleming** (919) 420-7660 Excellence Delivered As Promised NC Lic. No. F-0270



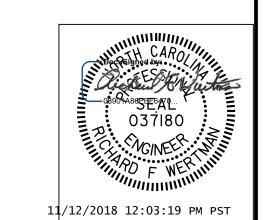
<u>PLAN</u>



BERM RIP RAPPED

ESTIMATED QUANTITIES							
BRIDGE @ STA.13+12.00 -L-	RIP RAP CLASS I (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE					
	TONS	SQUARE YARDS					
END BENT 1	35	40					
END BENT 2	65	70					

PROJECT NO. B-6023 HENDERSON \_ COUNTY STATION: 13+12.00 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH STANDARD

-RIP RAP DETAILS-

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 7/2/15 CHECKED BY: R.F. WERTMAN DATE: 7/3/15 MAA/GM MAA/GM MAA/THC DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84

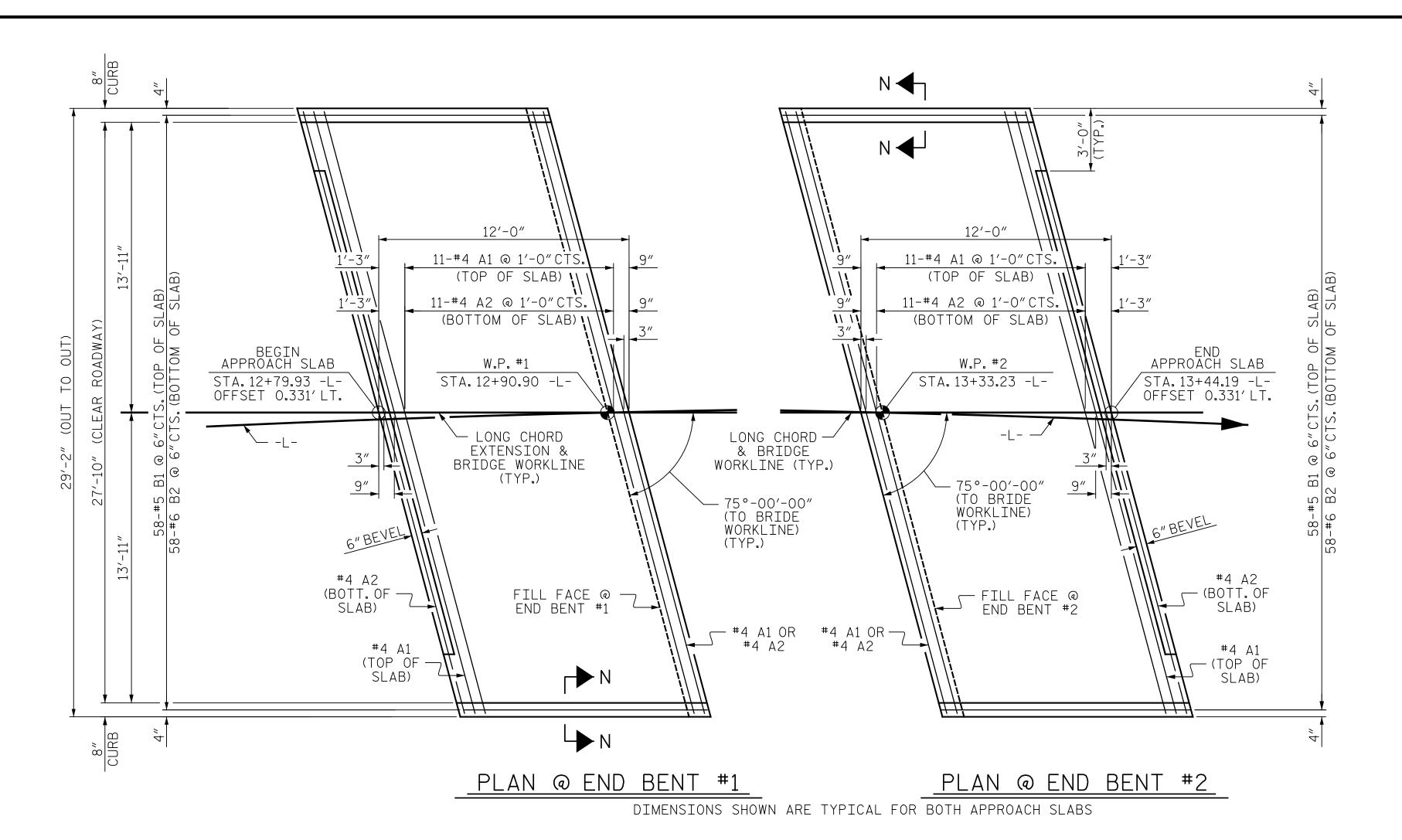


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DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

REVISIONS SHEET NO. NO. BY: S-13 DATE: DATE: TOTAL SHEETS

STR.#192 STD. NO. RR1



/- 5<sup>1</sup>/4" CONTINUOUS HIGH CHAIR UPPER (CHCU)

APPROXIMATE —

1: 1 SLOPE (TO BE DETERMINED

BY THE CONTRACTOR)

@ 3'-0"CTS. ACROSS SLAB

/-- #4 A1

4"Ø PERFORATED SCHEDULE 40 PVC PIPE

†2:1 SLOPE —

-GEOTEXTILE —

3'-0"

— SELECT

SECTION THRU SLAB

(TYPE II - MODIFIED APPROACH FILL)

MATERIAL

(CLASS V

OR CLASS VI)—

PROPOSED

ASPHALT PAVEMENT

#6 B2 —

† NORMAL TO END BENT

APPROVED WIRE BAR

SUPPORTS @ 3'-0"CTS.

ASSEMBLED BY: T.J.KIRSCHBAUM DATE: 12/22/14 CHECKED BY: R.F.WERTMAN DATE: 2/11/15

CHECKED BY: BCH 5-09 REV. 12-17 MAA/THC

DRAWN BY : SHS/MAA 5-09

ROADWAY

# NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

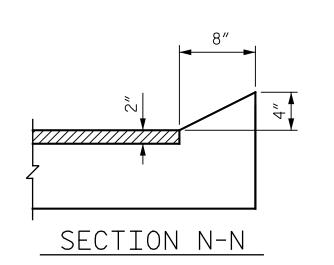
BILL OF MATERIAL							
AP	PRO	ACH	SLAI	B AT E	B #1		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
<b>*</b> A1	13	#4	STR	29'-10"	259		
Α2	13	#4	STR	29'-10"	259		
<b>★</b> B1	13	#5	STR	11'-1"	670		
B2	13	#6	STR	11'-7"	1009		
REINF	ORCI	NG STE	EL	LBS.	1268		
	* EPOXY COATED REINFORCING STEEL				929		
CLASS	SAA	CONCRE	TE	C. Y.	16.7		
AP	PRO	ACH	SLAE	B AT E	B #2		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
<b>*</b> A	13	#4	STR	29'-10"	259		
А	13	#4	STR	29'-10"	259		
* B	13	#5	STR	11'-1"	670		
В	13	#6	STR	11'-7"	1009		
REINF	ORCI	NG STE	EL	LBS.	1268		
	REINFORCING STEEL LBS. 1268  **EPOXY COATED REINFORCING STEEL LBS. 929						

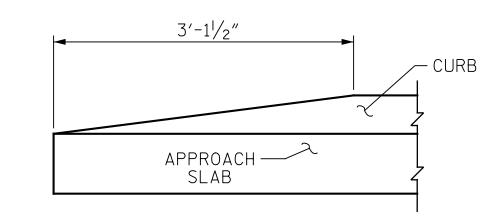
16.7

C.Y.

CLASS AA CONCRETE

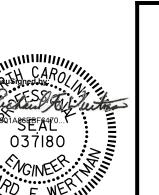
SPLICE LENGTHS				
BAR SIZE	EPOXY COATED	UNCOATE		
#4	2'-0"	1'-9"		
#5	2'-6"	2'-2"		
#6	3′-10″	2'-7"		





END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS



11/12/2018 12:03:19 PM PST

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PROJECT N	10. <u>B-</u> 6	5023
HEND	ERSON	COUNTY
STATION:_	13+12.0	<u> </u>

SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE

CORED SLAB UNIT (SUB-REGIONAL TIER) 75° SKFW

STATE OF NORTH CAROLINA

13 SKLW									
		SHEET NO.							
) <b>.</b>	BY:	DATE:	N0.	BY:	DATE:	S-14			
			<b></b>			TOTAL SHEETS			
			4			15			



SLAB

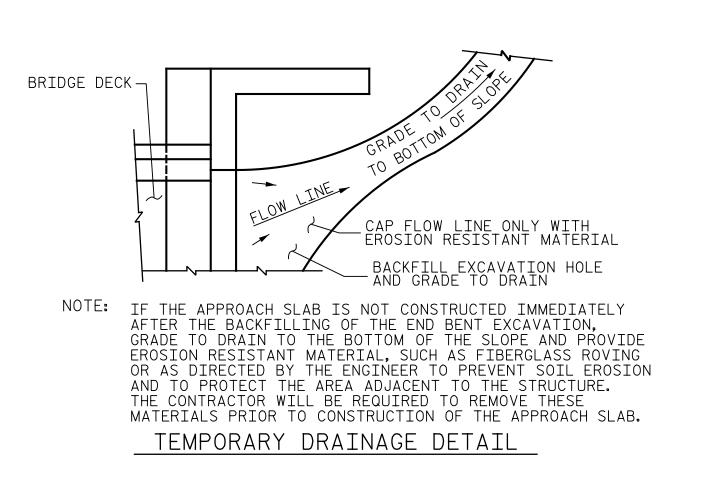
11/2" BACKER ROD

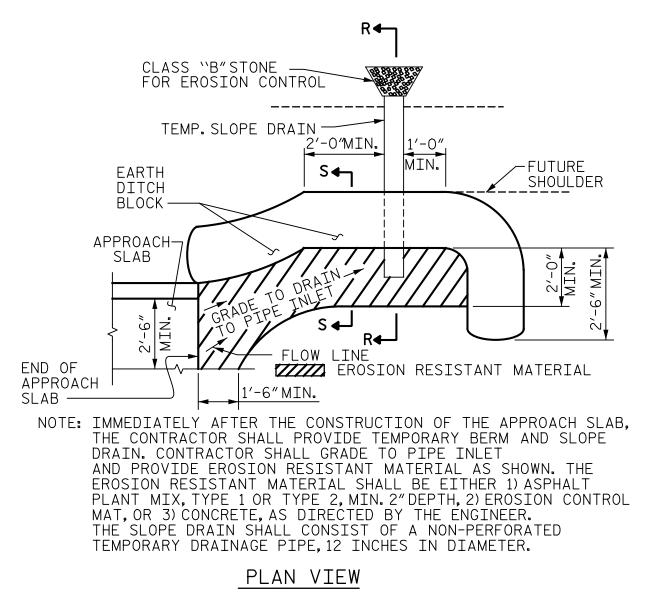
-2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND

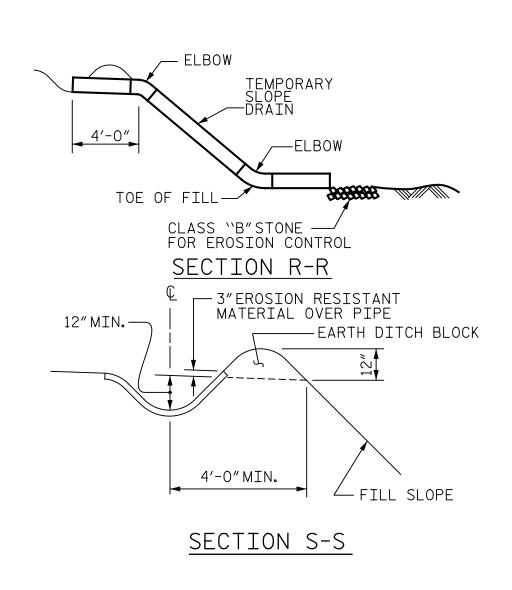
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ASSEMBLED BY: T.J.KIRSCHBAUM DATE: 12/22/14 CHECKED BY: R.F.WERTMAN DATE: 2/11/15

DRAWN BY: SHS/MAA 5-09 CHECKED BY: BCH 5-09 REV.12-17 MAA/THC





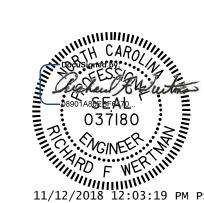


# TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

PROJECT NO. B-6023 HENDERSON STATION: 13+12.00 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER) 75° SKEW

DATE: BY:

STR.#192

**Gannett Fleming**Raleigh NC 27607-3073
(919) 420-7660 Excellence Delivered As Promised NC Lic. No. F-0270

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO. REVISIONS S-15 NO. BY: DATE: TOTAL SHEETS

# STANDARD NOTES

# DESIGN DATA:

# MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

EQUIVALENT FLUID PRESSURE OF EARTH ---- 30 LBS. PER CU. FT.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

# CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

# CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

# DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

# ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

# STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \( \frac{1}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

# HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

# SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

DocuSigned by:

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